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R403.1.3.5.4 Lap splices. Vertical and horizontal reinforcement shall be the longest lengths practical. Where splices are necessary in reinforcement, the length of lap splice shall be in accordance with Table R608.5.4.(1) and Figure R608.5.4(1). The maximum gap between noncontact parallel bars at a lap splice shall not exceed the smaller of one-fifth the required lap length and 6 inches (152 mm) [see Figure R608.5.4(1)].

R403.1.3.6 Isolated concrete footings. In detached one- and two-family dwellings that are three stories or less in height and constructed with stud bearing walls, isolated plain concrete footings supporting columns or pedestals are permitted.

R403.1.4 Minimum depth. All exterior footings and foundation systems shall extend 42 inches (1067 mm) below actual grade. Where applicable, the depth of the footings shall also conform to Section R403.1.4.1 of the code.

Exception: Upon evidence of the existence of any of the following conditions, the building official may modify the footing depth accordingly:

- (a) Freezing temperatures (freezing degree days).
- (b) Soil type.
- (c) Ground water conditions.
- (d) Snow depth experience.
- (e) Exposure to the elements.
- (f) Other specific conditions identified by the building official that may affect the foundation system.

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R403.1.4.1 Frost protection. Except where otherwise protected from frost, foundation walls, piers and other permanent supports of buildings and structures shall be protected from frost by one or more of the following methods:

- 1. Extended below the frost line specified in Table R301.2.(1).
- 2. Constructed in accordance with Section R403.3.
- 3. Constructed in accordance with ASCE 32.
- 4. Erected on solid rock.

Exceptions:

- 1. Protection of freestanding *accessory structures* with an area of 600 square feet (56 m^2) or less, of light-frame construction, with an eave height of 10 feet (3048 mm) or less shall not be required.
- 2. Protection of freestanding *accessory structures* with an area of 400 square feet (37 m^2) or less, of other than light-frame construction, with an eave height of 10 feet (3048 mm) or less shall not be required.
- 3. Decks not supported by a dwelling need not be provided with footings that extend below the frost line.

Footings shall not bear on frozen soil unless the frozen condition is permanent.

R403.1.5 Slope. The top surface of footings shall be level. The bottom surface of footings shall not have a slope exceeding one unit vertical in 10 units horizontal (10-percent slope). Footings shall be stepped where it is necessary to change the elevation of the top surface of the footings or where the slope of the bottom surface of the footings will exceed one unit vertical in 10 units horizontal (10-percent slope).

R403.1.6 Foundation anchorage. Wood sill plates and wood walls supported directly on continuous foundations shall be anchored to the foundation in accordance with this section.

Cold-formed steel framing shall be anchored directly to the foundation or fastened to wood sill plates anchored to the foundation. Anchorage of cold-formed steel framing and sill plates supporting cold-formed steel framing shall be in accordance with this section and Section R505.3.1 or R603.3.1.

Wood sole plates at all exterior walls on monolithic slabs, wood sole plates of *braced wall panels* at building interiors on monolithic slabs and all wood sill plates shall be anchored to the foundation with minimum $\frac{1}{2}$ -inchdiameter (12.7 mm) anchor bolts spaced a maximum of 6 feet (1829 mm) on center or approved anchors or anchor straps spaced as required to provide equivalent anchorage to $\frac{1}{2}$ -inch-diameter (12.7 mm) anchor bolts. Bolts shall extend a minimum of 7 inches (178 mm) into concrete or grouted cells of concrete masonry units. The bolts shall be located in the middle third of the width of the plate. A nut and washer shall be tightened on each anchor bolt. There shall be a minimum of two bolts per plate section with one bolt located not more than 12 inches (305 mm) or less than seven bolt diameters from each end of the plate section. Interior bearing wall sole plates on monolithic slab foundation that are not part of a braced wall panel shall be positively anchored with approved fasteners. Sill plates and sole plates shall be protected against decay and termites where required by Sections R317 and R318.

Exceptions:

- 1. Walls 24 inches (610 mm) total length or shorter connecting offset braced wall panels shall be anchored to the foundation with a minimum of one anchor bolt located in the center third of the plate section and shall be attached to adjacent braced wall panels at corners as shown in Item 9 of Table R602.3(1).
- 2. Connection of walls 12 inches (305 mm) total length or shorter connecting offset *braced wall panels* to the foundation without anchor bolts shall be permitted. The wall shall be attached to adjacent braced wall panels at corners as shown in Item 9 of Table R602.3(1).

R403.1.6.1 Foundation anchorage in Seismic Design Categories C, D_0, D_1 and D_2. In addition to the requirements of Section R403.1.6, the following

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SECTION R506 CONCRETE FLOORS (ON GROUND)

R506.1 General. Concrete slab-on-ground floors shall be designed and constructed in accordance with the provisions of this section or ACI 332. Floors shall be a minimum $3^{1}/_{2}$ inches (89 mm) thick (for expansive soils, see Section R403.1.8). The specified compressive strength of concrete shall be as set forth in Section R402.2.

R506.2 Site preparation. The area within the foundation walls shall have all vegetation, top soil and foreign material removed.

R506.2.1 Fill. Fill material shall be free of vegetation and foreign material. The fill shall be compacted to ensure uniform support of the slab, and except where *approved*, the fill depths shall not exceed 24 inches (610 mm) for clean sand or gravel and 8 inches (203 mm) for earth.

R506.2.2 Base. A 4-inch-thick (102 mm) base course consisting of clean graded sand, gravel, crushed stone, crushed concrete or crushed blast-furnace slag passing a 2-inch (51 mm) sieve shall be placed on the prepared subgrade where the slab is below *grade*.

Exception: A base course is not required where the concrete slab is installed on well-drained or sand-gravel mixture soils classified as Group I according to the United Soil Classification System in accordance with Table R405.1.

R506.2.3 Vapor retarder. A 6-mil (0.006 inch; 152 μ m) polyethylene or *approved* vapor retarder with joints lapped not less than 6 inches (152 mm) shall be placed between the concrete floor slab and the base course or the prepared subgrade where no base course exists.

Exception: The vapor retarder is not required for the following:

- 1. Garages, utility buildings and other unheated *accessory structures*.
- 2. For unheated storage rooms having an area of less than 70 square feet (6.5 m^2) and carports.
- 3. Driveways, walks, patios and other flatwork not likely to be enclosed and heated at a later date.
- 4. Where *approved* by the *building official*, based on local site conditions.

R506.2.4 Reinforcement support. Where provided in slabs-on-ground, reinforcement shall be supported to remain in place from the center to upper one-third of the slab for the duration of the concrete placement.

SECTION R507 EXTERIOR DECKS

R507.1 Decks. Wood-framed decks shall be in accordance with this section or Section R301 for materials and conditions not prescribed herein. Where supported by attachment to an exterior wall, decks shall be positively anchored to the primary structure and designed for both vertical and lateral loads.

Such attachment shall not be accomplished by the use of toenails or nails subject to withdrawal. Where positive connection to the primary building structure cannot be verified during inspection, decks shall be self-supporting. For decks with cantilevered framing members connections to exterior walls or other framing members shall be designed and constructed to resist uplift resulting from the full live load specified in Table R301.5 acting on the cantilevered portion of the deck.

R507.2 Deck ledger connection to band joist. Deck ledger connections to band joists shall be in accordance with this section, Tables R507.2 and R507.2.1, and Figures R507.2.1(1) and R507.2.1(2). For other grades, species, connection details and loading conditions, deck ledger connections shall be designed in accordance with Section R301.

R507.2.1 Ledger details. Deck ledgers installed in accordance with Section R507.2 shall be a minimum 2-inch by 8-inch (51 mm by 203 mm) nominal, pressure-preservative-treated southern pine, incised pressure-preservative-treated Hem-fir, or approved, naturally durable, No. 2 grade or better lumber. Deck ledgers installed in accordance with Section R507.2 shall not support concentrated loads from beams or girders. Deck ledgers shall not be supported on stone or masonry veneer.

R507.2.2 Band joist details. Band joists attached by a ledger in accordance with Section R507.2 shall be a minimum 2-inch-nominal (51 mm), solid-sawn, spruce-pine-fir lumber or a minimum 1-inch by $9^{1}/_{2}$ -inch (25 mm × 241 mm) dimensional, Douglas fir, laminated veneer lumber. Band joists attached by a ledger in accordance with Section R507.2 shall be fully supported by a wall or sill plate below.

R507.2.3 Ledger to band joist fastener details. Fasteners used in deck ledger connections in accordance with Table R507.2 shall be hot-dipped galvanized or stainless steel and shall be installed in accordance with Table R507.2.1 and Figures R507.2.1(1) and R507.2.1(2).

R507.2.4 Flashing. An approved corrosion-resistant flashing as required by Section R703.8 shall be installed above the attached ledger as shown in Figure R507.2.1(2) or as approved.

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R507.3 Plastic composite deck boards, stair treads, guards, or handrails. Plastic composite exterior deck boards, stair treads, guards and handrails shall comply with the requirements of ASTM D7032 and the requirements of Section 507.3.

R507.3.1 Labeling. Plastic composite deck boards and stair treads, or their packaging, shall bear a label that indicates compliance to ASTM D7032 and includes the allowable load and maximum allowable span determined in accordance with ASTM D7032. Plastic or composite handrails and guards, or their packaging, shall bear a label that indicates compliance to ASTM D7032 and includes the maximum allowable span determined in accordance with ASTM D7032.

R507.3.2 Flame spread index. Plastic composite deck boards, stair treads, guards, and handrails shall exhibit a flame spread index not exceeding 200 when tested in accor-

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dance with ASTM E84 or UL 723 with the test specimen remaining in place during the test.

Exception: Plastic composites determined to be non-combustible.

R507.3.3 Decay resistance. Plastic composite deck boards, stair treads, guards and handrails containing wood, cellulosic or other biodegradable materials shall be decay resistant in accordance with ASTM D7032.

R507.3.4 Termite resistance. Where required by Section 318, plastic composite deck boards, stair treads, guards and handrails containing wood, cellulosic or other biode-gradable materials shall be termite resistant in accordance with ASTM D7032.

507.3.5 Installation of plastic composites. Plastic composite deck boards, stair treads, guards and handrails shall be installed in accordance with this code and the manufacturer's instructions.

R507.4 Decking. Maximum allowable spacing for joists supporting decking shall be in accordance with Table R507.4. Wood decking shall be attached to each supporting member with not less than (2) 8d threaded nails or (2) No. 8 wood screws.

R507.5 Deck joists. Maximum allowable spans for wood deck joists, as shown in Figure R507.5, shall be in accordance with Table R507.5. Deck joists shall be permitted to cantilever not greater than one-fourth of the actual, adjacent joist span.

R507.5.1 Lateral restraint at supports. Joist ends and bearing locations shall be provided with lateral restraint to prevent rotation. Where lateral restraint is provided by joist hangers or blocking between joists, their depth shall equal not less than 60 percent of the joist depth. Where lateral restraint is provided by rim joists, they shall be secured to the end of each joist with not less than (3) 10d (3-inch \times 0.128-inch) nails or (3) No. 10 \times 3-inch (76 mm) long wood screws.

R507.6 Deck Beams. Maximum allowable spans for wood deck beams, as shown in Figure R507.6, shall be in accordance with Table R507.6. Beam plies shall be fastened with two rows of 10d (3-inch \times 0.128-inch) nails minimum at 16 inches (406 mm) on center along each edge. Beams shall be permitted to cantilever at each end up to one-fourth of the actual beam span. Splices of multispan beams shall be located at interior post locations.

DECK LEDGER CONNECTION TO BAND JOIST ^{a, b} (Deck live load = 40 psf, deck dead load = 10 psf, snow load ≤ 40 psf)									
	JOIST SPAN								
CONNECTION DETAILS	6' and less	6′1″ to 8′	8′1″ to 10′	10 [′] 1 [″] to 12 [′]	12'1" to 14'	14′1″ to 16′	16′1″ to 18′		
	On-center spacing of fasteners								
¹ / ₂ -inch diameter lag screw with ¹ / ₂ -inch maximum sheathing ^{c, d}	30	23	18	15	13	11	10		
$^{1/2}$ -inch diameter bolt with $^{1/2}$ -inch maximum sheathing ^d	36	36	34	29	24	21	19		
$^{1/2}$ -inch diameter bolt with 1-inch maximum sheathing ^e	36	36	29	24	21	18	16		

TABLE R507 2

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 pound per square foot = 0.0479 kPa.

a. Ledgers shall be flashed in accordance with Section R703.8 to prevent water from contacting the house band joist.

b. Snow load shall not be assumed to act concurrently with live load.

c. The tip of the lag screw shall fully extend beyond the inside face of the band joist.

d. Sheathing shall be wood structural panel or solid sawn lumber.

e. Sheathing shall be permitted to be wood structural panel, gypsum board, fiberboard, lumber or foam sheathing. Up to $\frac{1}{2}$ -inch thickness of stacked washers shall be permitted to substitute for up to $\frac{1}{2}$ inch of allowable sheathing thickness where combined with wood structural panel or lumber sheathing.

TABLE 507.2.1 PLACEMENT OF LAG SCREWS AND BOLTS IN DECK LEDGERS AND BAND JOISTS

MINIMUM END AND EDGE DISTANCES AND SPACING BETWEEN ROWS							
TOP EDGE BOTTOM EDGE ENDS ROW SPACING							
Ledger ^a	2 inches ^d	³ / ₄ inch	2 inches ^b	1 ⁵ / ₈ inches ^b			
Band Joist ^c	$^{3}/_{4}$ inch	2 inches	2 inches ^b	1 ⁵ / ₈ inches ^b			

For SI: 1 inch = 25.4 mm.

a. Lag screws or bolts shall be staggered from the top to the bottom along the horizontal run of the deck ledger in accordance with Figure R507.2.1(1).

b. Maximum 5 inches.

c. For engineered rim joists, the manufacturer's recommendations shall govern.

d. The minimum distance from bottom row of lag screws or bolts to the top edge of the ledger shall be in accordance with Figure R507.2.1(1).

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R507.7 Deck joist and deck beam bearing. The ends of each joist and beam shall have not less than $1^{1}/_{2}$ inches (38 mm) of bearing on wood or metal and not less than 3 inches (76 mm) on concrete or masonry for the entire width of the beam. Joist framing into the side of a ledger board or beam shall be supported by approved joist hangers. Joists bearing on a beam shall be connected to the beam to resist lateral displacement.

R507.7.1 Deck post to deck beam. Deck beams shall be attached to deck posts in accordance with Figure R507.7.1 or by other equivalent means capable to resist lateral displacement. Manufactured post-to-beam connectors shall be sized for the post and beam sizes. All bolts shall have washers under the head and nut.

Exception: Where deck beams bear directly on footings in accordance with Section R507.8.1.

R507.8 Deck posts. For single-level wood-framed decks with beams sized in accordance with Table R507.6, deck post size shall be in accordance with Table R507.8.

TABLE R507.8 DECK POST HEIGHT

DECK POST SIZE	MAXIMUM HEIGHT ^a
4×4	8'
4 × 6	8'
6 × 6	14'

For SI: 1 foot = 304.8 mm.

a. Measured to the underside of the beam.

R507.8.1 Deck post to deck footing. Posts shall bear on footings in accordance with Section R403 and Figure R507.8.1. Posts shall be restrained to prevent lateral displacement at the bottom support. Such lateral restraint

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shall be provided by manufactured connectors installed in accordance with Section R507 and the manufacturers' instructions or a minimum post embedment of 12 inches (305 mm) in surrounding soils or concrete piers.

TABLE R507.4 MAXIMUM JOIST SPACING

	MAXIMUM ON-CENTER JOIST SPACING					
MATERIAL TIPE AND NOMINAL SIZE	Perpendicular to joist	Diagonal to joist ^a				
$1^{1}/_{4}$ -inch-thick wood	16 inches	12 inches				
2-inch-thick wood	24 inches	16 inches				
Plastic composite	In accordance with Section R507.3	In accordance with Section R507.3				

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 degree = 0.01745 rad.

a. Maximum angle of 45 degrees from perpendicular for wood deck boards

TABLE R507.5 DECK JOIST SPANS FOR COMMON LUMBER SPECIES ^f (ft in.)								
SPECIES ^a	SIZE	SPACING OF DE	CK JOISTS WITH I (inches)	NO CANTILEVER ^b	SPACING OF DECK JOISTS WITH CANTILEVERS ^c (inches)			
		12	16	24	12	16	24	
	2×6	9-11	9-0	7-7	6-8	6-8	6-8	
Southern nine	2×8	13-1	11-10	9-8	10-1	10-1	9-8	
Southern prile	2×10	16-2	14-0	11-5	14-6	14-0	11-5	
	2×12	18-0	16-6	13-6	18-0	16-6	13-6	
Douglas fir-larch ^d ,	2×6	9-6	8-8	7-2	6-3	6-3	6-3	
	2×8	12-6	11-1	9-1	9-5	9-5	9-1	
spruce-pine-fir ^d	2×10	15-8	13-7	11-1	13-7	13-7	11-1	
	2×12	18-0	15-9	12-10	18-0	15-9	12-10	
Redwood, western cedars, ponderosa pine ^e ,	2×6	8-10	8-0	7-0	5-7	5-7	5-7	
	2×8	11-8	10-7	8-8	8-6	8-6	8-6	
	2×10	14-11	13-0	10-7	12-3	12-3	10-7	
rea pine	2×12	17-5	15-1	12-4	16-5	15-1	12-4	

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 pound per square foot = 0.0479 kPa, 1 pound = 0.454 kg.

a. No. 2 grade with wet service factor.

b. Ground snow load, live load = 40 psf, dead load = 10 psf, L/Δ = 360.

c. Ground snow load, live load = 40 psf, dead load = 10 psf, L/Δ = 360 at main span, L/Δ = 180 at cantilever with a 220-pound point load applied to end.

d. Includes incising factor.

e. Northern species with no incising factor

f. Cantilevered spans not exceeding the nominal depth of the joist are permitted.



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DECK BEAM SPAN LENGTHS ^{a, b} (ft in.)									
SPECIES	SIZE ^d	DECK JOIST SPAN LESS THAN OR EQUAL TO:							
SFECIES		6	8	10	12	14	16	18	
	$2-2 \times 6$	6-11	5-11	5-4	4-10	4-6	4-3	4-0	
	$2-2 \times 8$	8-9	7-7	6-9	6-2	5-9	5-4	5-0	
	$2 - 2 \times 10$	10-4	9-0	8-0	7-4	6-9	6-4	6-0	
Southam ning	$2 - 2 \times 12$	12-2	10-7	9-5	8-7	8-0	7-6	7-0	
Southern pine	$3-2 \times 6$	8-2	7-5	6-8	6-1	5-8	5-3	5-0	
	$3-2 \times 8$	10-10	9-6	8-6	7-9	7-2	6-8	6-4	
	$3 - 2 \times 10$	13-0	11-3	10-0	9-2	8-6	7-11	7-6	
	$3 - 2 \times 12$	15-3	13-3	11-10	10-9	10-0	9-4	8-10	
	$3 \times 6 \text{ or } 2 - 2 \times 6$	5-5	4-8	4-2	3-10	3-6	3-1	2-9	
	$3 \times 8 \text{ or } 2 - 2 \times 8$	6-10	5-11	5-4	4-10	4-6	4-1	3-8	
	$3 \times 10 \text{ or } 2 - 2 \times 10$	8-4	7-3	6-6	5-11	5-6	5-1	4-8	
Douglas fir-larch ^e	$3 \times 12 \text{ or } 2 - 2 \times 12$	9-8	8-5	7-6	6-10	6-4	5-11	5-7	
hem-fir ^e ,	4 × 6	6-5	5-6	4-11	4-6	4-2	3-11	3-8	
spruce-pine-fir ^e ,	4×8	8-5	7-3	6-6	5-11	5-6	5-2	4-10	
western cedars,	4×10	9-11	8-7	7-8	7-0	6-6	6-1	5-8	
ponderosa pine ^f ,	4 × 12	11-5	9-11	8-10	8-1	7-6	7-0	6-7	
red pine	$3-2 \times 6$	7-4	6-8	6-0	5-6	5-1	4-9	4-6	
	$3-2 \times 8$	9-8	8-6	7-7	6-11	6-5	6-0	5-8	
	$3 - 2 \times 10$	12-0	10-5	9-4	8-6	7-10	7-4	6-11	
	$3 - 2 \times 12$	13-11	12-1	10-9	9-10	9-1	8-6	8-1	

TABLE R507.6 DECK BEAM SPAN I ENGTHS^{a, b} (ft - ir

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 pound per square foot = 0.0479 kPa, 1 pound = 0.454 kg.

a. Ground snow load, live load = 40 psf, dead load = 10 psf, L/Δ = 360 at main span, L/Δ = 180 at cantilever with a 220-pound point load applied at the end.

b. Beams supporting deck joists from one side only.

c. No. 2 grade, wet service factor.

d. Beam depth shall be greater than or equal to depth of joists with a flush beam condition.

e. Includes incising factor.

f. Northern species. Incising factor not included.



FIGURE R507.6 TYPICAL DECK BEAM SPANS

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For SI: 1 inch = 25.4 mm.

FIGURE R507.7.1 DECK BEAM TO DECK POST



FIGURE R507.8.1 TYPICAL DECK POSTS TO DECK FOOTINGS

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